

## Technical data sheet

**SIMPSON****Strong-Tie**

POLY-GP

### General Purpose Resin Mortar

Chemical anchor for use in concrete and masonry. Specially formulated for light or medium duty fixing into hollow or solid base materials. Poly-GP300 is easy to use and fast curing, it enables good performance when used in applications such as fixing architectural steel work, cable trays, hand rails and gates.

## Features

### Material

- Styrene free polyester
- Use with Simpson Strong-Tie threaded rod (LMAS) : galvanised steel and stainless A4-70

### Benefits

- Fast curing.
- Non-flammable.
- Low odour.
- Colour changes when cured.

## Applications

### Header member

**Hollow or solid masonry.**

### For Use With

- Blinds
- Hinges
- Air conditioners
- Satellite Dishes
- Boilers



Fixation d'une cloture

## POLY-GP General Purpose Resin Mortar

## Technical Data

### Références

References	Product information						
	DB nr.	NOBB nr.	Grey color	Beige color	Content [ml]	Weight [kg]	Packaging qty [pcs]
POLYGP300GB-PL	-	-	-	x	300	0.586	12
POLYGP420B-PL	-	-	-	x	420	0.842	12

### Design resistance – Tension – NRd [kN] – Carbon steel 5.8

References	Design resistance – NRd – Carbon steel 5.8 [kN]							
	Non-cracked concrete							
	hef = 8d				hef = 12d			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
POLY-GP + LMAS M8	4.6	5	5.3	5.5	6.9	7.4	7.9	8.2
POLY-GP + LMAS M12	10	10.9	11.6	12	15.1	16.3	17.3	17.9
POLY-GP + LMAS M16	14.3	15.4	16.4	17	21.4	23.2	24.7	25.5

#### Concrete :

1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing  $s \geq 15$  cm (any diameter) or with a rebar spacing  $s \geq 10$  cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges ( $c \leq \max [10 \text{ hef}; 60d]$ ) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is  $\sigma_L + \sigma_R \leq 0$ . In the absence of detailed verification  $\sigma_R = 3 \text{ N/mm}^2$  can be assumed ( $\sigma_L$  equals the tensile stress within the concrete induced by external loads, anchors loads included).

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Design resistance – Tension –  $N_{Rd}$  [kN] – Stainless steel A4-70

References	Design resistance – $N_{Rd}$ – Stainless steel A4-70 [kN]							
	Non-cracked concrete							
	$h_{ef} = 8d$				$h_{ef} = 12d$			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
POLY-GP + LMAS M8	4.6	5	5.3	5.5	6.9	7.4	7.9	8.2
POLY-GP + LMAS M12	10	10.9	11.6	12	15.1	16.3	17.3	17.9
POLY-GP + LMAS M16	14.3	15.4	16.4	17	21.4	23.2	24.7	25.5

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Design resistance – Shear –  $V_{Rd}$  [kN] – Carbon steel 5.8

References	Design resistance – $V_{Rd}$ – Carbon steel 5.8 [kN]							
	Non-cracked concrete							
	$h_{ef} = 8d$				$h_{ef} = 12d$			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
POLY-GP + LMAS M8	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
POLY-GP + LMAS M12	16.8	16.8	16.8	16.8	16.8	16.8	16.8	16.8
POLY-GP + LMAS M16	31.2	31.2	31.2	31.2	31.2	31.2	31.2	31.2

Concrete :

1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing  $s \geq 15$  cm (any diameter) or with a rebar spacing  $s \geq 10$  cm, if the rebar diameter is 10mm or smaller.
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Design resistance – Shear –  $V_{Rd}$  [kN] – Stainless steel A4-70

References	Design resistance – $V_{Rd}$ – Stainless steel A4-70 [kN]							
	Non-cracked concrete							
	$h_{ef} = 8d$				$h_{ef} = 12d$			
	C20/25	C30/37	C40/50	C50/60	C20/25	C30/37	C40/50	C50/60
POLY-GP + LMAS M8	8.3	8.3	8.3	8.3	8.3	8.3	8.3	8.3
POLY-GP + LMAS M12	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2
POLY-GP + LMAS M16	34.3	34.3	34.3	34.3	35.3	35.3	35.3	35.3

Concrete :

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3. Concrete is considered non-cracked when the tensile stress within the concrete is  $\sigma_L + \sigma_R \leq 0$ . In the absence of detailed verification  $\sigma_R = 3$  N/mm<sup>2</sup> can be assumed ( $\sigma_L$  equals the tensile stress within the concrete induced by external loads, anchors loads included).

Design resistance – Bending moment –  $M_{Rd}$  [Nm]

References	Design resistance – Bending moment – $M_{Rd}$ [Nm]	
	Carbon steel 5.8	Stainless steel A4-70
POLY-GP + LMAS M8	15.2	16.7
POLY-GP + LMAS M12	52.8	59
POLY-GP + LMAS M16	133.6	149.4

Concrete :

1. The design loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s). The loading figures are valid for unreinforced concrete and reinforced concrete with a rebar spacing  $s \geq 15$  cm (any diameter) or with a rebar spacing  $s \geq 10$  cm, if the rebar diameter is 10mm or smaller.
2. The figures for shear are based on a single anchor without influence of concrete edges. For anchorages close to edges ( $c \leq \max [10 h_{ef}; 60d]$ ) the concrete edge failure shall be checked per ETAG 001, Annex C, design method A.
3. Concrete is considered non-cracked when the tensile stress within the concrete is  $\sigma_L + \sigma_R \leq 0$ . In the absence of detailed verification  $\sigma_R = 3$  N/mm<sup>2</sup> can be assumed ( $\sigma_L$  equals the tensile stress within the concrete induced by external loads, anchors loads included).

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Design resistance –  $h_{ef} = 80 \text{ mm}$  ( $\leq M8$ ) or  $85 \text{ mm}$  ( $\geq M10$ ) – Carbon steel  $\geq 4.6$  /  
Stainless steel  $\geq A2-70$

References	Design resistance – Carbon steel $\geq 4.6$ / stainless steel $\geq A2-70$			
	$h_{ef} = 80 \text{ mm}$ ( $\leq M8$ ) or $85 \text{ mm}$ ( $\geq M10$ )			
	Tension - $N_{Rd}$ [kN]		Shear - $V_{Rd}$ [kN]	
	Solid Clay Masonry	Hollow Masonry	Solid Clay Masonry	Hollow Masonry
POLY-GP + LMAS M6	1.6	0.8	2.4	0.8
POLY-GP + LMAS M8	1.6	0.8	2.4	0.8
POLY-GP + LMAS M12	1.6	0.8	2.8	0.8

## Masonry :

	Compressive strength $f_b$ [N/mm <sup>2</sup> ]	Bulk density $\rho$ [kg/m <sup>3</sup> ]
Solid clay masonry	$\geq 18$	$\geq 1600$
Hollow masonry	$\geq 6$	$\geq 900$

1. The design resistances have been calculated using the partial safety factors for resistances stated in ETA-approval(s).
2. The recommended loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s) and with a partial safety factor for actions of  $\gamma_F=1.4$ .
3. For combined tension and shear loads or anchor groups and/or in case of edge influence, a calculation acc. TR 054, design method A shall be performed. For details see ETA - assessment(s)
4. Temperature range:  $-40^\circ\text{C}/+40^\circ\text{C}$  ( $T_{mlp} = +24^\circ\text{C}$ )
5. Coefficient factor  $\beta$  for in situ tests acc. ETAG 029 see ETA-19/0642; Annex C2
6. Displacements under service load see ETA-19/0642; Annex C2 & C3

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Design resistance – Bending moment –  $M_{Rd}$  [Nm]

References	Design resistance – Bending moment – $M_{Rd}$ [Nm]		
	Carbon steel 5.8	Carbon steel 8.8	Stainless steel $\geq$ A2-70
POLY-GP + LMAS M6	6.4	9.6	7.1
POLY-GP + LMAS M8	15.2	24	16.7
POLY-GP + LMAS M12	52.8	84	59

## **Masonry :**

	Compressive strength $f_b$ [N/mm <sup>2</sup> ]	Bulk density [kg/m <sup>3</sup> ]
Solid clay masonry	$\geq 18$	$\geq 1600$
Hollow masonry	$\geq 6$	$\geq 900$

1. The design resistances have been calculated using the partial safety factors for resistances stated in ETA-approval(s).
2. The recommended loads have been calculated using the partial safety factors for resistances stated in ETA-approval(s) and with a partial safety factor for actions of  $\gamma_F=1.4$ .
3. For combined tension and shear loads or anchor groups and/or in case of edge influence, a calculation acc. TR 054, design method A shall be performed. For details see ETA - assessment(s)
4. Temperature range: -40°C/+40°C ( $T_{mlp} = +24^\circ\text{C}$ )
5. Coefficient factor  $\beta$  for in situ tests acc. ETAG 029 see ETA-19/0642; Annex C2
6. Displacements under service load see ETA-19/0642; Annex C2 & C3

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## Installation

### Curing Schedule

Temperature of the anchorage base $T_{\text{base material}}$	Working time (Gel time) $t_{\text{gel}}$	Curing time (in dry concrete) $t_{\text{cure, dry}}$	Curing time (in wet concrete) $t_{\text{cure, wet}}$
$0^{\circ}\text{C} \leq T_{\text{base material}} < +10^{\circ}\text{C}$	20 min	90 min	3:00 h
$+10^{\circ}\text{C} \leq T_{\text{base material}} < +20^{\circ}\text{C}$	9 min	60 min	2:00 h
$+20^{\circ}\text{C} \leq T_{\text{base material}} < +30^{\circ}\text{C}$	5 min	30 min	1:00 h
$+20^{\circ}\text{C} \leq T_{\text{base material}} \leq +40^{\circ}\text{C}$	3 min	20 min	40 min

- **Manual Air Cleaning (MAC)** for all drill hole diameters  $d_0 \leq 24$  mm and drill holl depth  $h_0 \leq 10d$  :  
 4x blowing (hand pump)  
 4x brushing (twisting motion)  
 4x blowing (Hand pump)
- **Compressed Air Cleaning (CAC)** for all drill hole diameters  $d_0$  and drill hole depths :  
 2x blowing (min. 6 bar - oil free compressed air)  
 2x brushing  
 2x blowing (min. 6 bar - oil free compressed air)
- **Cartridge temperature (Bond material)** :  $\geq +20^{\circ}\text{C}$

# Technical data sheet

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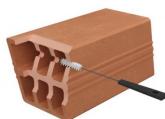
**Strong-Tie**

POLY-GP

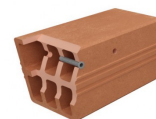
**General Purpose Resin Mortar**



*Drill.*



*Brush.*



*Insert sieve.*



*Inject the resin.*



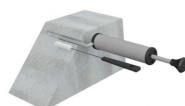
*Insert the rod, turning slowly.*



*Once set, full load capacity is reached.*



*Drill.*



*Remove dust by brushing and blowing.*



*Fill the hole to half or two thirds, Withdrawing the nozzles with each pump.*



*Insert the rod, turning slowly.*



*Once set, full load capacity is reached.*

## Installation parameters – Concrete

References	Installation parameters - Concrete					
	Ø drilling [d <sub>0</sub> ] [mm]	Max. fixture hole Ø [d <sub>f</sub> ] [mm]	Drilling depth (8d) [h <sub>0</sub> =h <sub>ef</sub> =8d] [mm]	Drilling depth (12d) [h <sub>0</sub> =h <sub>ef</sub> =12d] [mm]	Wrench size [SW]	Installation torque [T <sub>inst</sub> ] [Nm]
POLY-GP + LMAS M8	10	9	64	96	13	8
POLY-GP + LMAS M12	14	14	96	144	19	15
POLY-GP + LMAS M16	18	18	128	192	24	25

## Spacing, edge distances and member thickness – Concrete

References	Spacing, edge distance and member thickness - Concrete											
	Effective embedment depth (8d) [h <sub>ef,8d</sub> ] [mm]	Characteristic spacing for h <sub>ef,8d</sub> [S <sub>cr,N</sub> ] [mm]	Characteristic edge distance for h <sub>ef,8d</sub> [c <sub>cr,N</sub> ] [mm]	Min. member thickness for h <sub>ef,8d</sub> [h <sub>min</sub> ] [mm]	Effective embedment depth (12d) [h <sub>ef,12d</sub> ] [mm]	Characteristic spacing for h <sub>ef,12d</sub> [S <sub>cr,N</sub> ] [mm]	Characteristic edge distance for h <sub>ef,12d</sub> [c <sub>cr,N</sub> ] [mm]	Min. member thickness for h <sub>ef,12d</sub> [h <sub>min</sub> ] [mm]	Min. spacing [S <sub>min</sub> ] [mm]		Min. edge distance [C <sub>min</sub> ] [mm]	
									8d	12d	8d	12d
POLY-GP + LMAS M8	64	192	96	100	96	288	144	126	32	48	32	48
POLY-GP + LMAS M12	96	288	144	126	144	432	216	174	48	72	48	72
POLY-GP + LMAS M16	128	384	192	158	192	576	288	222	64	96	64	96



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## Installation parameters – Masonry – Solid clay masonry

References	Installation parameters - Solid clay masonry			
	Ø drilling [d <sub>0</sub> ] [mm]	Max. fixture hole Ø [d <sub>f</sub> ] [mm]	Drilling depth [h <sub>1</sub> ] [mm]	Installation torque [T <sub>inst</sub> ] [Nm]
POLY-GP + LMAS M6	8	7	85	2
POLY-GP + LMAS M8	10	9	85	2
POLY-GP + LMAS M12	14	14	90	2

## Installation parameters – Masonry – Hollow masonry

References	Installation parameters - Hollow masonry			
	Ø drilling [d <sub>0</sub> ] [mm]	Max. fixture hole Ø [d <sub>f</sub> ] [mm]	Drilling depth [h <sub>1</sub> ] [mm]	Installation torque [T <sub>inst</sub> ] [Nm]
POLY-GP + LMAS M6	12	7	85	1.5
POLY-GP + LMAS M8	12	9	85	1.5
POLY-GP + LMAS M12	16	14	90	1.5

## Spacing, edge distances and member thickness – Masonry – Solid clay masonry

References	Spacing, edge distance and member thickness - Solid clay masonry			
	Min. spacing [S <sub>min</sub> ] [mm]			Min. edge distance [C <sub>min</sub> ] [mm]
	S <sub>cr,N</sub> = S <sub>min</sub>	S <sub>cr,N</sub>    = S <sub>min</sub>	S <sub>cr,N</sub> <sup>T</sup> = S <sub>min</sub> <sup>T</sup>	C <sub>cr,N</sub> = C <sub>min</sub>
POLY-GP + LMAS M6	240	-	-	120
POLY-GP + LMAS M8	240	-	-	120
POLY-GP + LMAS M12	255	-	-	127.5

## Spacing, edge distances and member thickness – Masonry – Hollow masonry

References	Spacing, edge distance and member thickness - Hollow masonry			
	Min. spacing [S <sub>min</sub> ] [mm]			Min. edge distance [C <sub>min</sub> ] [mm]
	S <sub>cr,N</sub> = S <sub>min</sub>	S <sub>cr,N</sub>    = S <sub>min</sub>	S <sub>cr,N</sub> <sup>T</sup> = S <sub>min</sub> <sup>T</sup>	C <sub>cr,N</sub> = C <sub>min</sub>
POLY-GP + LMAS M6	-	250	120	100
POLY-GP + LMAS M8	-	250	120	100
POLY-GP + LMAS M12	-	250	120	100

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